BOI TUP®

Job Notes Example Problem 1. Figure 4. Double shear. Sides S4S 2x10, Main S4S 3x10 3/4" A307 bolts @ 3" o.c. in row. 2 rows, 3 per row angle load to grain: main member = 0, side member = 0 snow load No.. 1 Doug Fir-Larch

Fill in yellow cells below
Fill in turquoise cells if needed
Red cells are calculated
Light green cells are tables

Except for the yellow and turquoise cells, this spreadsheet is protected to prevent accidental loss of the formulas.

specific gravity (g) or dowel bearing (b)		g
single shear (s) or double shear (d)		d
side members wood (w) or steel (s)		W
main members wood (w) or masonry (m)		W
bolt diameter (inches) =) =	0.75
dowel bending yield (psi) = F_{yi}	ь =	45000
side member thickness (inches) = L_s	=	1.50
main member thickness (inches) = L _{rr}	. =	2.50
side member specific gravity = G _s	; =	0.5
main member specific gravity = G _n	, =	0.5
load to grain side member 0 =	=	0
load to grain main member ° =	, =	0

side member dowel b	earing	=	F_{es}
side member dowel b	earing	=	F_{es}
side member dowel b	earing	=	F_{es}
main member dowel b	earing	=	Fem
main member dowel b	earing	=	Fem
main member dowel b	earing	=	Fem
_			
used F _{es} =	5600		

1 R_t =

Load Duration Factors,C_D (Ref.3)



k ₁ =	0.5815
k ₂ =	1.3338
k ₃ =	1.8316
K =	1

Yield Mode I _m Z =	2630 pounds
Yield Mode I _s Z =	3150 pounds
Yield Mode II Z =	N.A. pounds
Yield Mode III _m Z =	N.A. pounds
Yield Mode IIIs Z =	2400 pounds
Yield Mode IV Z =	3220 pounds

BASIC DESIGN VALUE	2400 pounds
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Number of Bolts in Joint =	6
Load Duration Factor, $C_D =$	1.15
Wet Conditions Factor, $C_M =$	1.00
Temperature Factor, C _t =	1.00
Bolt Group Factor, $C_g =$	1.00

occupancy live	1
snow	1.15
construction	1.25
wind, quake	1.6

used F_{em} =

Bolt Gro	up Action	Factor Information	
number o	f rows =		2
number b	olts in a row	=	3
bolt spaci	ng in row (in	ches) =	3.0
main men	nber mod. of	elast (x106 psi) =	1.7
side mem	ber mod. of	elast (x106 psi) =	1.6
main men	nber gross a	rea (sq. in.) =	23.12
side mem	ber gross ar	ea (sq. in.) =	27.75
ë =	180000		
u =	1.00841		
	0.07040		

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Design Value per Bolt = 2760 pounds
Total all Bolts = 16490 pounds

NET SECTION CALCULATIONS FOR ALLOWABLE TENSION LOAD

9.25 in 9.25 in 19.06 s
19.06 s
22.88 s
tef.4)
factor
1.5
1.4
1.3
1.2
1.1
1.0
1.0
2

R_{EA} =

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